

# The influence of reclaimed asphalt pavement in warm mix asphalt on asphalt concrete binder course with Retona Blend 55

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**Abstract.** Reclaimed asphalt pavement (RAP) comprises removed pavement materials containing high-quality aggregates and asphalt which can be recycled as materials for new pavement construction. It is removed continually for reconstruction, resurfacing, and maintenance purposes, and if not recycled will become waste. This paper determines the influence of using different RAP percentages and asphalt content in warm mix asphalt on the Marshall test results for asphalt concrete binder course (AC-BC) using Retona Blend 55. The percentages of RAP are determined by analyzing the gradation of the existing aggregates in RAP and adding virgin aggregates so that it meets the standard gradation for AC-BC specified by the Ministry of Public Works and Housing. The RAP percentages in the asphalt mixes in this study are 35%, 45%, and 51.55% of total aggregates, while the asphalt contents are 5%, 6%, and 7% of the total mix. To determine the influence of RAP percentage and asphalt content, and to discover if there is any influence from the interaction between these two factors, the analysis is performed using a factorial design. The results of this study show that variation in RAP percentages in the mix has no significant influence on stability, flow, and Marshall quotient, but there is significant influence on void in mineral aggregates (VMA), void in mix (VIM), and void filled with asphalt (VFA). Correlations of 97.5%, 80%, and 95.1%, respectively show that increase in RAP percentage increases VMA and VIM and decreases VFA. The interaction between RAP percentage and asphalt content has no significant influence on Marshall test results.

## 1 Introduction

Flexible pavement is the most widely used type of pavement. In 2016, the length of paved roads in Indonesia was 287,926 km and the length of such roads in the country is increasing yearly [1]. The frequent use of flexible pavement causes demand for aggregates and asphalt. Both materials are non-renewable, so the use of virgin materials will continuously diminish the supply of these two resources. Alternative sources such as recycled asphalt are therefore needed to reduce the use of new materials [2].

Asphalt mixing in Indonesia is still often done at hot temperatures, known as ‘hot mix asphalt’ (HMA). In this study asphalt mixing was carried out at warm temperatures, known as ‘warm mix asphalt’ (WMA), a method that is more environmentally friendly [3].

## 2 Literature Review

### 2.1 Reclaimed asphalt pavement (RAP)

RAP is a layer of asphalt that is peeled from the pavement for road repairs or to access utilities under the road surface. RAP contains asphalt and aggregates which can be reused in new road construction [2]. RAP has been widely used as a material for the manufacture of flexible

pavements in America and Europe, and more than 99% of asphalt is recycled in America [4]. The use of RAP not only reduces the use of new materials and prevents pollution from asphalt disposal but also reduces construction costs [2,4].

### 2.2 Warm mix asphalt (WMA)

WMA is a technology that allows mixing and processing of asphalt mixtures at temperatures of up to 20-30 °C, i.e. much lower than for HMA [5]. For a lower mixing temperature, less fuel is used, thus reducing the greenhouse gas emissions produced in asphalt mixture production. WMA method with a mixing temperature of 36.5° C (i.e. lower than for HMA), resulted in emissions of SO<sub>2</sub> decreased by 83%, NO<sub>x</sub> by 31%, CO by 62%, and VOCs by 63% [6].

In addition to fuel savings and emissions reduction, WMA has other benefits, such as ease of working (because of lower temperatures) that increases worker productivity. Lower temperatures also allow longer processing time. Moreover, WMA uses the same mixing and compaction equipment as HMA, so WMA technology is very easy to implement [2,3,4].

The lower mixing temperature is obtained by additives to the asphalt mixture. These additives reduce the viscosity of asphalt or increase its volume, enabling the asphalt to effectively envelop the aggregate at lower

temperatures [7]. In this study, the additives used were zeolites, which are non-metallic minerals formed from volcanic sedimentation and found in Indonesia [8]. These minerals are hydrated porous alumina silicate crystals with a three-dimensional skeleton structure formed from tetrahedral  $[SiO_4]$  4- and  $[AlO_4]$  5-. Zeolite decreases the viscosity of asphalt by creating a foam effect [9].

### 3 Materials and Research Methods

This research is a quantitative study with an experimental method. The data used in this study were obtained from laboratory test results. This study aims to find out the influence of RAP percentage and asphalt content in the mixture on the Marshall test results for the WMA-RAP in the AC-BC layer. In order to determine the influence of these two factors the experiment is done using factorial design [10]. And to determine the significance of the influence, two-way analysis of variance (ANOVA) is performed using SPSS [11].

The main material used in the asphalt mixture was RAP. This material had been peeled from the existing road and was taken to the laboratory for extraction and tested for asphalt content and aggregate gradation. Once both were known, could determine how much virgin asphalt and virgin aggregates that need to be added to the mixture. The asphalt used in this study is Retona Blend 55. It is a blend of petroleum asphalt and natural rock asphalt from Buton island, Indonesia [12].

The variation in RAP aggregates percentage were 51.55%, 45%, and 35% of total aggregates, and the variation in asphalt content were 5%, 6%, and 7%. For each group 3 specimens were made. Therefore there were 27 specimens in total. Then Marshall tests were carried out to determine stability, flow, Marshall quotient, void in mineral aggregates, void in mix and void filled with asphalt.

## 4 Results and Analysis

### 4.1 Reclaimed asphalt pavement

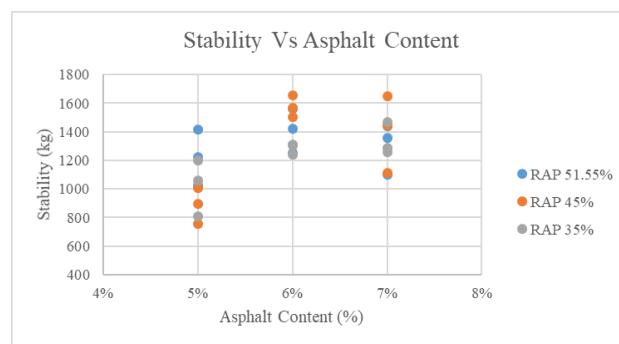
In this study, 12 extractions were carried out and it was found that the RAP consisted of 5.807% asphalt and 94.193% aggregates. RAP asphalt characteristics can be seen in Table 1. After a long-term aging, asphalt becomes hard and less workable [13]. It is indicated by low penetration, low ductility and high softening point.

**Table 1.** RAP asphalt.

Test	Test methods	Results
Penetration at 25 °C (0.1 mm)	SNI 2456-2011	8.73
Softening point (°C)	SNI 2434-2011	72
Ductility at 25 °C (mm)	SNI 2432-2011	42
Flash point (°C)	SNI 2433-2011	240
Specific gravity	SNI 2441-2011	1.061

### 4.2 Marshall test results

#### 4.2.1 Stability



**Fig. 1.** Stability vs. asphalt content.

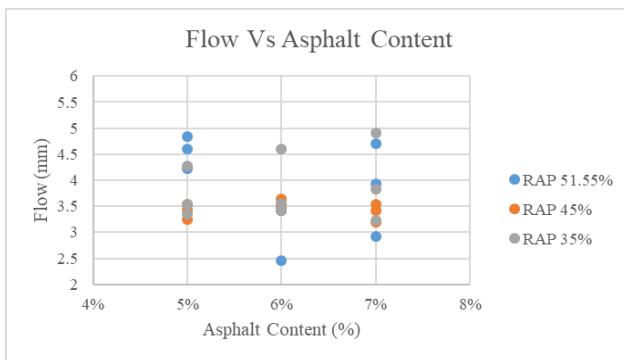
Stability is the maximum load that can be held by the asphalt mixture. Figure 1 shows the relationship between stability and asphalt content in the asphalt mixture for each mixture with RAP aggregate percentage of 51.55%, 45%, and 35%. From the graph it can be seen that in the samples with RAP of 51.55% and 45% the highest stability value obtained was from the sample with 6% asphalt content. Whereas in the sample with 35% RAP aggregate, the highest stability value was obtained from samples with 7% asphalt content. This could be due to the asphalt mixture with 35% RAP aggregate containing more virgin aggregate. RAP aggregates have been mixed with asphalt while new aggregates have not been mixed with asphalt. With more virgin aggregates, more aggregate surfaces have not been covered with asphalt so more virgin asphalt is needed. Therefore the 7% asphalt mixture in samples with 35% RAP aggregates has a higher stability than samples with 5% and 6% asphalt content.

Tests of Between-Subjects Effects					
Dependent Variable:	Stability				
Source	Type III Sum of Squares	df	Mean Square	F	Sig.
Corrected Model	1041174.660*	8	130146.833	4.823	.003
Intercept	43676868.170	1	43676868.170	1618.559	.000
RAP Percentage	43709.683	2	21854.841	.810	.460
Asphalt Content	732817.223	2	366408.612	13.578	.000
RAP Percentage * Asphalt Content	264647.754	4	66161.939	2.452	.083
Error	485730.462	18	26985.026		
Total	45203773.290	27			
Corrected Total	1526905.122	26			

**Fig. 2.** ANOVA factorial design test results using SPSS (stability).

Figure 2 shows that the significance of the influence of RAP percentage is 0.46. This figure is greater than 0.05, so the influence of RAP percentages on the stability of asphalt mixtures is not significant. The significance of the influence of asphalt content was 0. This means that 100% asphalt content affects the stability of the asphalt mixture. While the significance of the interaction influence of the two factors is 0.083. This means that the interaction influence of the two factors on the stability of asphalt mixture is not significant.

#### 4.2.2 Flow



**Fig. 3.** Flow vs. asphalt content.

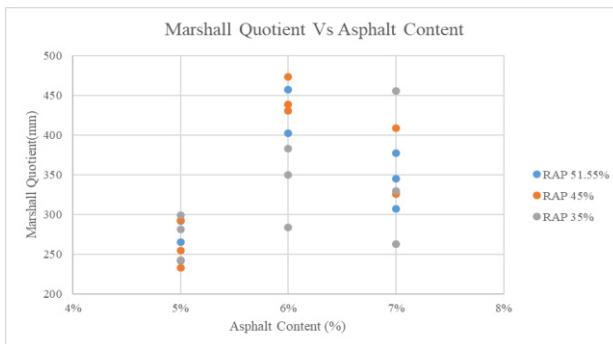
Flow is the amount of deformation that occurs in the asphalt mixture when it collapses. Figure 3 shows the relationship between flow and asphalt content for each mixture with RAP aggregate of 51.55%, 45%, and 35%. In the sample with 45% and 35% RAP, asphalt content does not strongly affect flow. In samples with RAP aggregate content of 51.55%, the highest flow was obtained from samples with 6% asphalt content.

Tests of Between-Subjects Effects					
Source	Type III Sum of Squares	df	Mean Square	F	Sig.
Corrected Model	4.189 <sup>a</sup>	8	.524	1.755	.153
Intercept	372.076	1	372.076	1247.446	.000
RAP Percentage	1.025	2	.513	1.719	.207
Asphalt Content	.690	2	.345	1.157	.337
RAP Percentage * Asphalt Content	2.473	4	.618	2.073	.127
Error	5.369	18	.298		
Total	381.633	27			
Corrected Total	9.557	26			

**Fig. 4.** ANOVA factorial design test results using SPSS (flow).

From the ANOVA test in Figure 4, the significance of the influence of RAP percentage, asphalt content, and the interaction of RAP and asphalt content were 0.207, 0.337, and 0.127, respectively. All three are greater than 0.05, which means that these influences on the flow of the asphalt mixture are not significant.

#### 4.2.3 Marshall quotient



**Fig. 5.** Marshall quotient vs. asphalt content.

Figure 5 shows the relationship between Marshall quotient (MQ) and asphalt content for mixtures with RAP

of 51.55%, 45%, and 35%. From the graph, it can be seen that in the samples with 51.55% and 45% RAP, the highest MQ occurred in samples with 6% asphalt content. This is because in the sample with 51.55% and 45% RAP aggregate the highest stability occurred in samples with 6% asphalt content, while the flow was not much different in the 5%, 6%, and 7% samples. In the sample with 35% RAP aggregate, the highest stability occurred in samples with 7% asphalt content and there was no difference in the flow of samples with asphalt content of 5%, 6%, and 7%.

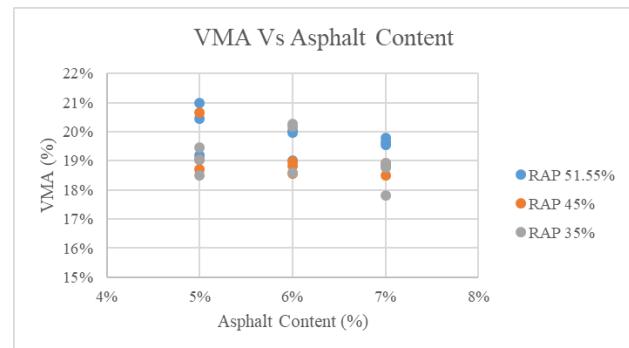
#### Tests of Between-Subjects Effects

Source	Type III Sum of Squares	df	Mean Square	F	Sig.
Corrected Model	139154.590 <sup>a</sup>	8	17394.324	5.543	.001
Intercept	3314798.021	1	3314798.021	1056.324	.000
RAP Percentage	13455.071	2	6727.535	2.144	.146
Asphalt Content	103080.313	2	51540.157	16.424	.000
RAP Percentage * Asphalt Content	22619.206	4	5654.802	1.802	.172
Error	56484.928	18	3138.052		
Total	3510437.539	27			
Corrected Total	195639.518	26			

**Fig. 6.** ANOVA factorial design test results using SPSS (Marshall quotient).

Figure 6 shows that the significance of the influence of RAP percentage and the interaction influence of RAP percentages and asphalt content are 0.146 and 0.172. This means that these two factors have no significant influence on the Marshall asphalt mixture quotient, while the significance of the influence of asphalt content is 0. This means that the influence of asphalt content is very significant for the Marshall quotient.

#### 4.2.4 Void in mineral aggregates (VMA)



**Fig. 7.** VMA vs. asphalt content.

VMA is the volume of cavity between the aggregate particles to the total volume. From Figure 7 it can be seen in the samples with RAP aggregate of 51.55% and 45% the greater the asphalt content the smaller the VMA value. In samples with asphalt content of 5% and 7%, the greater the percentage of aggregate RAP in the mixture, the greater the VMA value. This is because the greater the percentage of RAP aggregate, the greater the RAP asphalt content of the total asphalt used in the mixture. RAP asphalt is hard and brittle and has lower workability. With more workability, the mixture is more easily

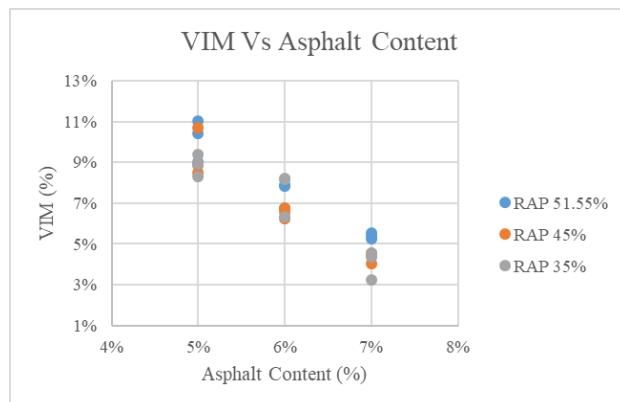
blended therefore more aggregate surface is covered with asphalt.

Tests of Between-Subjects Effects					
Dependent Variable: VMA					
Source	Type III Sum of Squares	df	Mean Square	F	Sig.
Corrected Model	7.613 <sup>a</sup>	8	.952	2.212	.077
Intercept	10059.350	1	10059.350	23377.510	.000
RAP Percentage	3.932	2	1.966	4.569	.025
Asphalt Content	1.648	2	.824	1.915	.176
RAP Percentage * Asphalt	2.033	4	.508	1.181	.352
Content					
Error	7.745	18	.430		
Total	10074.708	27			
Corrected Total	15.359	26			

**Fig. 8.** ANOVA factorial design test results using SPSS (VMA).

From the results of the ANOVA test for VMA shown in Figure 8, the significance of the influence of RAP percentages, asphalt content, and the interaction of RAP percentages and asphalt content are 0.025, 0.176, and 0.352. This means that the influence of asphalt content and the interaction of both have no influence on asphalt VMA. However, RAP percentages have a significant influence on VMA with a correlation of 97.5%.

#### 4.2.5 Void in mix (VIM)



**Fig. 9.** VIM vs. asphalt content.

VIM is the total volume of air between asphalt-coated aggregate particles. From Figure 9 it can be seen that for the three percentages of RAP aggregate, the value of VIM decreases with increasing levels of asphalt in the mixture. This is because the more asphalt used, the more cavities are filled with asphalt so the total air in the mixture decreases. In samples with asphalt content of 5% and 7%, samples with a higher percentage of RAP aggregate had a higher VIM. This is because the nature of the RAP asphalt is hard, so it is less able to cover the aggregate surface and so fewer cavities are filled with asphalt. Therefore, the higher the content of RAP asphalt in the mixture, the greater the air volume in the mixture.

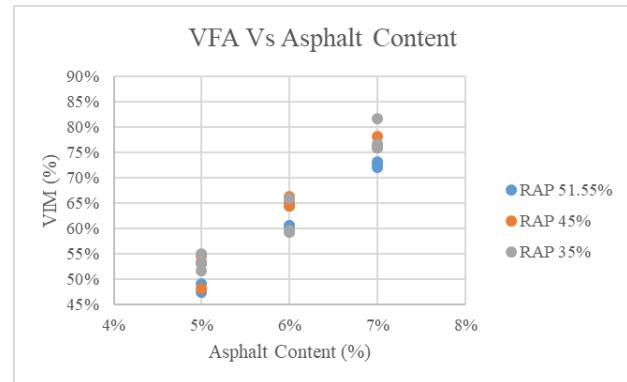
#### Tests of Between-Subjects Effects

Source	Type III Sum of Squares	df	Tests of Between-Subjects Effects		
			Mean Square	F	Sig.
Corrected Model	374.004 <sup>a</sup>	8	46.751	4.309	.005
Intercept	1908.900	1	1908.900	175.943	.000
RAP Percentage	106.062	2	53.031	4.888	.020
Asphalt Content	159.323	2	79.661	7.342	.005
RAP Percentage * Asphalt	108.620	4	27.155	2.503	.079
Content					
Error	195.291	18	10.850		
Total	2478.196	27			
Corrected Total	569.296	26			

**Fig. 10.** ANOVA factorial design test results using SPSS (VIM).

Figure 10 shows the significance of the influence of RAP percentages and asphalt content and the interaction of both are 0.02, 0.005, and 0.079. From these results it is evident that percentage of RAP and asphalt content have a significant influence on VIM, with a correlation of 80% and 99.5% respectively. The interaction of RAP percentages and asphalt content, however, does not have a significant influence on VIM.

#### 4.2.6 Void filled with asphalt (VFA)



**Fig. 11.** VFA vs. asphalt content.

Figure 11 shows that for the three percentages of RAP aggregate, the greater the asphalt content in the mixture, the greater the VFA. This is because the more asphalt used, the more cavities in the mixture are filled with asphalt. Therefore, VFA is inversely proportional to VIM. For samples with asphalt content of 5% and 7%, the greater the aggregate percentage of RAP, the smaller the VFA value. The greater the percentage of RAP aggregate in the mixture, the greater the RAP asphalt content is than the total asphalt so that the asphalt becomes harder. Therefore, with the same asphalt content, asphalt mixtures with higher levels of RAP asphalt are less able to cover the aggregate surface, causing less cavities to be filled with asphalt.

Tests of Between-Subjects Effects					
Dependent Variable: VFA	Type III Sum of Squares	df	Mean Square	F	Sig.
Corrected Model	2714.387*	8	339.298	56.068	.000
Intercept	108924.144	1	108924.144	17999.388	.000
RAP Percentage	58.417	2	29.209	4.827	.021
Asphalt Content	2620.156	2	1310.078	216.486	.000
RAP Percentage*Asphalt Content	35.814	4	8.953	1.480	.250
Error	108.928	18	6.052		
Total	111747.459	27			
Corrected Total	2823.315	26			

**Fig. 12.** ANOVA factorial design test results using SPSS (VFA).

The results of the ANOVA test in Figure 12 show the significance of the influence of RAP percentage and asphalt content, and the interaction of the two factors is 0.021, 0.000, and 0.25. This means that the percentage of RAP and asphalt content have a significant influence on VFA with a correlation of 97.9% and 100%, respectively. The interaction of the two factors, however, does not have a significant influence on the VFA.

## 5 Conclusion

- RAP used in this study consists of 5.807% asphalt and 94.193% aggregate.
- The results of the RAP asphalt characteristics test are penetration of 8.7, softening point of 72 °C, and ductility of 42 mm. RAP asphalt is brittle and hard causing it to have very low workability. In future experiments asphalt rejuvenation should be performed or more virgin asphalt should be added to the mixture to increase workability. Therefore producing asphalt mixture with better characteristics.
- The RAP aggregate percentage in the mixture has a significant influence on VMA, VIM, and VFA.
- Asphalt content in the mixture has a significant influence on stability, Marshall quotient, VIM, and VFA.
- The interaction of RAP percentages and asphalt content did not have a significant influence on the Marshall test results.

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## References

1. Badan Statistik Indonesia [Statistics Indonesia]. (1957-2016). *Panjang jalan menurut jenis permukaan* [Road length based on surface type]. Retrieved from <http://www.bps.go.id/linkTableDinamis/view/id/820>.

2. Federal Highway Administration. (2011). *Reclaimed asphalt pavement in asphalt mixtures: state of the practice*. Retrieved from <http://www.fhwa.dot.gov/publications/research/infras tructure/pavements/11021/11021.pdf>
3. H.A. Rondón-Quintana, J.A. Hernández-Noguera, and F.A. Reyes-Lizcano, *Ingeniería e Investigación* vol. 35 n.º 3, december - 2015 (5-18). DOI:<http://dx.doi.org/10.15446/ing.investig.v35n3.50463>
4. National Asphalt Pavement Association. (2015). *6<sup>th</sup> annual survey: recycled materials and warm-wix asphalt usage 2015*.
5. N. Wahjuningsih, R.J. Sumabrata, S.P. Hadiwardoyo, E3S Web of Conferences **34**, 01032 (2018)
6. Ohio Research Institute for Transportation and the Environment. (2009). *Performance assessment of marm mix asphalt (WMA) pavements*. Retrieved from [https://rosap.ntl.bts.gov/view/dot/16978/dot\\_16978\\_DS1.pdf?](https://rosap.ntl.bts.gov/view/dot/16978/dot_16978_DS1.pdf)
7. M. Zaumanis, V. Haritonovs,. (2010). *Research on properties of warm mix asphalt*, Scientific Journal of Riga Technical University, Construction Science, Vol 11, 77–84.
8. Kusdarto. (2008). *Potensi zeolit di Indonesia [Zeolit potential in Indonesia]*. *Jurnal Zeolit Indonesia*, **7**(2), 78-87.
9. A. Woszuk, W. Franus, *A review of the application of zeolite materials in warm mix asphalt technologies* (2017)
10. N. Lathifah, *Analisis pengaruh penambahan BNA-R pada aspal dengan metode faktorial desain* [Analysis of influence of BNA-R addition in asphalt using factorial experimental design method]. (Undergraduate thesis, 2017). Unpublished.
11. A. Field, *Discovering statistics using SPSS*. 3<sup>rd</sup> edition. (2009)
12. Sumarji, *Penggunaan aspal Buton tipe retona blend 55 sebagai bahan susun campuran HRS-B* [The use of Buton asphalt type retona blend 55 as a component of HRS-B mix]. *Jurnal Teknik*, **2**(1), 18-24
13. L.X. Dai, *Evaluation of warm mix asphalt performance incorporating high reclaimed asphalt pavement content* (ME Thesis, University of Canterbury, 2016)