

Conducting tensometric monitoring of the technical and stress-strain state of underground facilities

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Abstract. The authors describe the experience of carrying out a scientific and technical research study as part of scientific and technical support at the construction and design stages of large-scale construction projects in the city of Moscow (including unique ones). Analytical data are presented as a result of studying the stressed-deformed state of a pile foundation with step-by-step application of load in the course of implementing civil construction projects.

1 Introduction

When designing buildings and structures based on the current regulatory framework (including [1]), the developers of design documentation take into account a wide range of safety factors in order to ensure the regulator compliant safety and reliability levels specified for a given object (see. Table 1).

Table 1. Safety factors according to the SP 20.13330.2011 Loads and Effects.

No.	Safety factor name as per Russian state standard SP 20.13330.2011 Loads and Effects	Safety factor designation as per SP 20.13330.2011 Loads and Effects
1.	Partial safety for materials	γ_m
2.	Partial safety for loads	γ
3.	Partial safety for work conditions	γ_c
4.	Partial safety the facility level of responsibility	K_n
5.	Partial safety for intended purpose reliability	γ_n
6.	Partial safety for ground	γ_g

Using the abovementioned factors is one of the basic features of the calculation of buildings and structures using the two limiting conditions method (see. Table. 2). This method is widely used today for design calculations (including verification calculations). However, the existing legal and technical basis clearly defines the procedure for definition

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of the cross sections dimensions of bearing elements of the buildings frame and the foundations, concrete reinforcement, location of necessary reinforcing elements (eg, visible and hidden capitals, strengthening of the columns punching areas, etc.) based on a strength condition, rigidity and stability. This results in the fact that it is now always possible to achieve the optimum components sizes. First of all, it relates to the economic feasibility (economy) issues as well as labor and material consumption in the design and construction of unique objects (see. Table. 3, [3]). Obviously, the regulatory and technical basis governing the construction design and production is not able to include all possible features of buildings and structures, as well as the unique features of the building conditions used in practice, despite frequent updates. It is impossible to achieve a maximum classification of objects by type in the conditions where development of creative engineering leads to a creation of unique objects, each having its own specific features (including architecture, structural elements, engineering systems, location, etc.).

Table 2. Calculation of building structures on modern methods of two limit states.

No.	Limit state	Calculation description
1	The first limit state	Determining the bearing capacity of the structure component (strength and resistance for compressed members)
2	The second limit state	Determining the maximum admissible deformation based on the smooth functioning condition in accordance with a predetermined purpose and physiological and aesthetic requirements

Table 3. Signs of especially dangerous, technically complex and unique buildings and structures in accordance with an applicable regulatory and technical base.

No.	Facility category	Facilities included in the class	Reference
1.	Especially dangerous	1. Nuclear facilities, waterworks,	Town Planning Code of the Russian Federation Chapter 6. Architectural and structural design, construction, reconstruction of capital construction (articles 47 to 55) Article 48.1. Especially dangerous, technically complex and unique objects
2.	Technically complex	2. communication facilities, 3. power lines and other electric grid facilities with voltage of 330 kV or more, 4. space facilities, 5. air transport facilities, 6. capital construction facilities of 7. common use railways infrastructure, 8. subways, 9. port waterworks, 10. thermal power plants, 11. air cableways, 12. other hazardous industrial facilities.	
3.	Unique facilities	1. height over 100 meters; 2. passages over 100 meters; 3. presence of a console with a length above 20 meters; 4. burying an underground part (partially or fully) more than 15 meters below the land planning level.	

Currently, the design of such unique facilities is performed based on a similar experience and using reliable solutions to the unique facilities construction that may be compared and correlated in terms of their the technical and structural characteristics. The similarity theory is not always able to provide optimized production solutions. The design

companies do not always perform analysis properly, so the investor of a construction has to bear additional costs arising from redundancy in the designing of buildings and structures.

Let us show the desire to optimize the sections of bearing components and consequently to reduce the consumption of materials and the estimated cost of construction on an example of authors' practical experience in monitoring of the actual stress-strain state of the foundation piles of the large unique facility in the city of Moscow. The purpose of this monitoring is to measure the actual stress tensor and its correlation with the calculated values obtained in the course of designing by the General designer company based on similar objects (see Table 4.).

Table 4. Purposes of the stress-strain state monitoring of concrete pile foundation of a unique object.

No.	Task category	Result
1.	Construction control	Ensuring the safety and reliability of structures; timely reaction in case of a critical value (limiting condition)
2.	Financial management	Using the bearing capacity latent reserves of the structural components; reduction of materials consumption; positive economic effect
3.	Scientific organization of labor (SOL)	Accumulation and systematization of knowledge about the actual behavior of a material structure under load, taking into account the building loading steps; updating the legal framework for the unique objects construction; drawing up recommendations for the design and construction of unique objects grouped by similarity (representative objects)

2 Materials and methods

In the considered object, full-bodied-reinforced concrete end-bearing piles 110.30-11u with square section 300x300 mm (section 1) and 100.40-11u size 400x400 mm (section 6) are applied. The procedure for sequential installation of TZB stress monitoring sensor in the pile body (depending on load) is shown in Table. 5.

Table 5. Procedure of sequential installation of TZB stress monitoring sensor in the pile body (depending on load).

No.	Operations	Photos from the scientific and technical support facility
1.	A hole of 80mm diameter and 400mm depth is drilled in the pile surface using a core barrel (hole size varies depending on the strain gauge size)	
2.	A tensometer is installed through the bottom of the hole levelled by special mixture	

No.	Operations	Photos from the scientific and technical support facility
3.	A repair mixture having strength in a design time not less than 100% of the concrete pile design class (400x400mm - B30; 300x300mm - B40.) is used for the hole concreting. The tensometer vertical position is visually controlled during concreting. To prevent damage to the power supply wire, it is placed in a metal sleeve.	

Before carrying out tests on a construction site, a number of preparatory operations should be carried out. To convert units of stress arising under the influence of the design loads in the concrete body to the units of mass, calibration dependence is determined by testing the concrete test samples with installed tensometer.

Sample piles of 600 mm are used as control samples (Fig. 1, a)) for the tensometer installation. Specially manufactured samples with dimensions 300x300x300mm are used as control samples for the tensometer installation.

The sensor concreting is performed during the pylon (wall) structure concreting with a tensometer installed using identical concrete mix. This provides the control sample hardening will be the same as hardening of the whole structure.



Fig. 1. A) Preparation of control sample test. B) Test of control samples using a hydraulic press.

Control samples tests are carried out using a hydraulic press (Fig. 1, b)). The readings obtained on the weighing transducer are considered indirect characteristic, and values of load (kN) on a test press shall be direct characteristic

3 Results and Discussion

The load on control samples is exerted step-by-step. The values of direct and indirect characteristics are fixed after regular time periods. Based on these data, the calibration dependence is determined (Fig. 3) allowing the load (kN) of stresses occurring in the concrete body to be correlated.

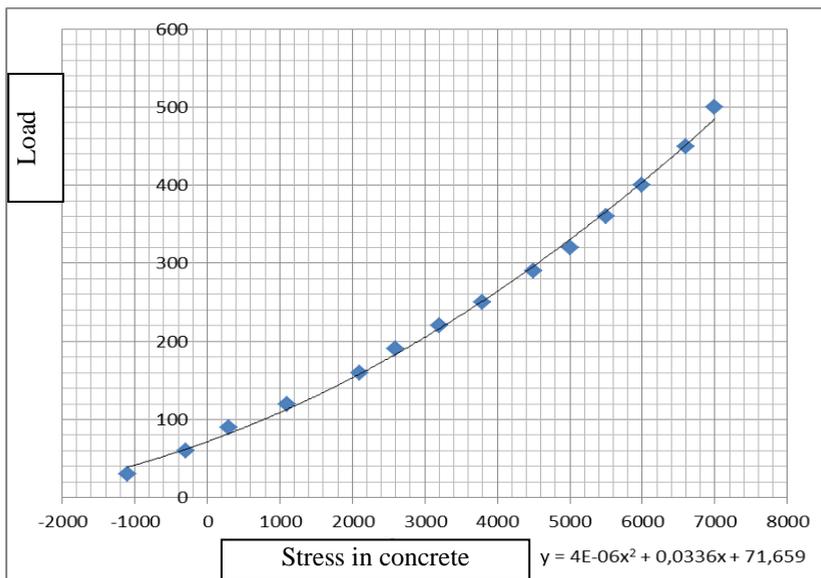


Fig. 3. The calibration dependence built based on the results of the control samples testing.

Table 6. Stages of measuring equipment readings obtaining and post-processing of the results of scientific and technical support of the construction and design of a unique facility.

No.	Stages of measuring equipment readings obtaining and post-processing of the results of scientific and technical support of the construction and design of a unique facility
	obtaining readings and calculation using a calibration dependence at following stages of building carcass loading
1.	- Initial measurement (after hardening)
2.	- Measurements after concreting the foundation plate
3.	- Measurements after the underground part construction
4.	- Measurements after construction of 50% of reinforced concrete aboveground structures
5.	- Measurements after construction of 100% of reinforced concrete aboveground structures
6.	-Measurements after construction of interior walls and partitions, facades (after completion of construction works on the examined sections)
7.	Analytical studies of the stress tensor in accordance with the design (calculated) data
8.	Formation of conclusions and recommendations for further safe and reliable operation of constructed structures, including making recommendations to reduce the consumption of materials in the construction of structures of similar objects

Table 7. Description of the equipment used. Concrete embedded tensometer (TZB 100; TZB-200).

No.	Characteristics of the equipment used	Value
1.	Measuring tensometer base (reduced length, mm).	100; 200
2.	Range of measured stress, kg/cm ²	300
3.	Tensometer sensitivity (mV/V) / (kg/cm ²)	0,032
4.	Tensometer accuracy,%	-2 +
5.	Operating temperatures, °C	-30 + 60
6.	Overload, % Of the NPI	50
7.	Input electrical resistance, Ohm	355.5
8.	Output electric resistance, Ohm	350.2
9.	DC supply voltage, not more than, V	12.0
10.	Power consumption, not more than, VA	0.4
11.	Insulation resistance, MOhm, not less	1000
12.	Protection class according to GOST 14254 (IEC 529-89)	IP65

4 Conclusion

To improve the quality of the construction supervision in the unique facilities construction, a set of measures of scientific and technical support described in this article can be applied. The strain studies described were performed within the framework of this support. Despite the fact that the importance and relevance of this kind of scientific research for the construction of unique objects is obvious, the modern legislation does not clearly regulate the contents and scope of scientific research during the scientific and technical support of construction and design (STSC). Timely monitoring of the state of the unique facilities considers interests of both the end user receiving the output quality finished building product and the initial investor, significantly reducing material consumption for the construction. The design companies obtain a very important control over the actual state of the structure during construction and will be able to response to possible emergency situations caused by design errors, legal framework imperfection in terms of design calculations of unique facilities construction and other destructive factors.

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