

Use of Stress Wave to Evaluate The Repair Quality of Concrete Crack

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Abstract. The objective of this paper is to develop a nondestructive test technique based on stress wave propagation for evaluating the repair quality of concrete. Concrete structures frequently exist various types of cracks due to inferior environment and excessive loading. There are several crack repairing technique used in concrete construction. Epoxy-injection is the one used mostly in crack repairing. In this paper, a newly developed technique based on the stress wave was used to evaluate the repair quality of concrete crack. Numerical results show that the stress wave propagation of the impact-echo tests is capable of identifying the infill of epoxy-injection in the crack of repaired structures. The new technique under development has a high potential for evaluation of the quality of the repaired cracks.

1 INTRODUCTION

There is an increasing need for structural repairing to extend the service life of deteriorated concrete structures. The cracks of concrete structure are the important indicator for the structure health diagnosis. Because the development of strong epoxy adhesive, epoxy resin injection becomes the most popular method of structural repairing for reinforced concrete members in the repair industry. The repairing method which injects Epoxy resin into the crack of concrete members is relatively simple and cost-effective, increases the strength of existing concrete members, blocks moisture penetration of reinforced concrete members and reduces the chance of corrosion.

The most important factor in determination of the repairing quality is the injection condition of the crack. Epoxy adhesives are used as an injection material to repair cracks but air voids at the crack will reduce the repair strength. To ensure the repairing quality, the techniques able to detect air voids of the crack after concrete members been repaired are needed. In this paper, a newly developed technique based on the Time-of-Flight Diffraction Technique was used to evaluate the repair quality of concrete crack. The research will started from numerical simulation of the stress wave propagation.

2 TIME-OF-FLIGHT DIFFRACTION TECHNIQUE

To measure the depth of surface-opening cracks in concrete, two receivers are used and located on the opposite sides of the crack. Figs. 1(a) shows the instrument arrangement. The first receiver is located on the impact side and has a distance H_0 to the impact point. The second receiver is located on the opposite side of the crack. Figs. 1(b) and (c) show typical waveforms recorded at the first and second receivers, respectively. After impact, the R-wave arrival at the first receiver will trigger the monitoring system. If the arrival time, t_1 ,

and the speed, C_R , of R-wave are known, the time of impact initiation can be calculated as $(t_1 - H_0/C_R)$. The value of t_1 is negative because a pre-trigger mode is used in recording signals to avoid losing important information. The region behind the crack cannot be penetrated by waves generated by impact until diffraction at the crack tip occurs. Thus, the second receiver initially responds to the arrival of the diffracted P-wave. The arrival time, t_2 , of the diffracted P-wave can be obtained by identifying the initial disturbance in the waveform recorded at the second receiver. Thus, the shortest travel time for P-wave from the impact point to the second receiver can be obtained as

$$\Delta t = t_2 - (t_1 - H_0/C_R) = t_2 - t_1 + H_0/C_R \quad (1)$$

Knowing the P-wave speed, C_p , in concrete, the shortest travel length of P-wave from the impact point to the second receiver can be calculated as $C_p \times \Delta t$. If the distances from the crack to the impact point and the second receiver are H_1 and H_2 respectively, the depth, d , of the vertical surface-opening crack can be determined by the following equation:

$$d = \sqrt{\left[\frac{(C_p \times \Delta t)^2 + H_1^2 - H_2^2}{2 \times C_p \times \Delta t} \right]^2 - H_1^2} \quad (2)$$

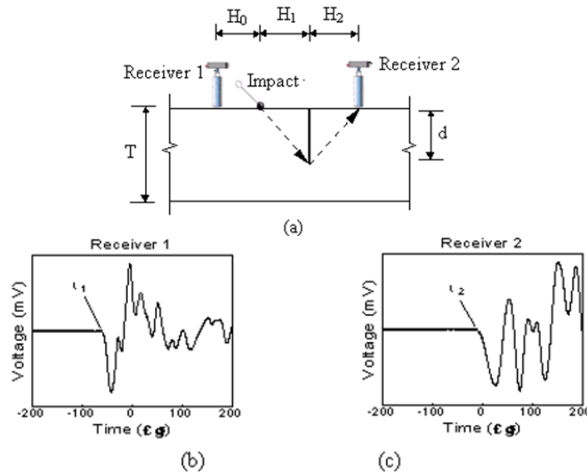


Fig. 1. Test scheme: (a) instrument arrangement; (b) waveform 1; (c) waveform 2.

3 NUMERICAL SIMULATION

In this paper, numerical simulations of the stress wave responses of crack were carried out using the explicit finite-element program LS-DYNA. All cases are simulated using 2-D axisymmetric, 4-node elements and isotropic models. Concrete is modeled as an isotropic material with a Poisson's ratio 0.2 and a density 2300kg/m^3 . A Young's modulus (E) of 33.1 GPa is chosen to provide a P-wave speed of 4000 m/s, the material parameters of Epoxy resin and air used in numerical simulation are also shown in Table 1. The time-history of nodal vertical displacements were recorded every $1.334 \mu\text{s}$; and 1024 data were recorded in each simulation.

Table 1. Material parameters of numerical simulation

Material	Wave speed C_p (m/s)	Density (kg/m ³)	Elastic modulus (pa)	Acoustic Impedance $Z, (C_p * \rho)$
Air	340	1.2	-	408
Epoxy Resin	2400	1141	5.602E9	2738400
Concrete	4000	2300	33.10E9	9200000

3.1 Numerical simulation of the time-of-flight diffraction with variable of different contact time

The correctness of the numerical simulation is carried out in this section. In numerical simulation model, the depth of the crack is 10 cm, the shape of force-time functions are half-sin which is used for introducing stress wave into numerical model as impactor. The distance of impact is setting at 4 cm(H_1) beside the crack. The time-history of nodal vertical displacements were recorded at the distance of 4 cm(H_2) located on the opposite side of the crack. Three force-time functions with different contact times as 10 μ s, 20 μ s, and 30 μ s are chosen to perform the numerical model.

The results of time-history of nodal vertical displacements with different contact time as 10 μ s, 20 μ s, and 30 μ s are shown in Fig.2. The nodal vertical displacements correspond to the arrival of the diffracted P-wave and the arrival time(t_2) of the diffracted P-wave is the same, 55 μ s, for different contact time(10 μ s, 20 μ s, and 30 μ s). According the results of numerical simulation, the depth of the crack can be calculated by using Eq.(2) as 9.95 cm. Comparing with 10 cm, the error percentage of numerical simulation results is below 1%, so the numerical simulation was proved to be valid.

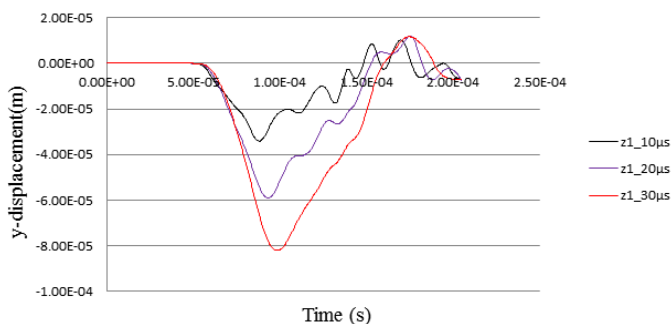


Fig. 2. waveform of time-of-flight diffraction with different contact-time

3.2 Numerical simulation of different injection condition of crack repairing

This section examines the stress wave propagation after the epoxy resin material filled in the crack with different partly filled scenarios. In numerical simulation, the depth of crack is also setting to 10 cm and the contact time of force-time function is 20 μ s in all cases. The variables of different epoxy resin injection scenarios in numerical simulations are shown in Table 2 and Fig. 3. For all the models, the epoxy is filled from the top surface and the air pack of the crack is underneath the epoxy.

Table 2. Variables of different epoxy resin injection scenarios

Code	Length of Epoxy Resin (cm)	Length of Air (cm)	Remark
E0 cm_A10 cm	0	10	Non- epoxy injection
E2 cm_A8 cm	2	8	Partial epoxy injection
E4cm_A6 cm	4	6	Partial epoxy injection
E6 cm_A4 cm	6	4	Partial epoxy injection
E8 cm_A2 cm	8	2	Partial epoxy injection
E10 cm_A0 cm	10	0	Fully epoxy injection
E0 cm_C10 cm	0	-	Non-crack case

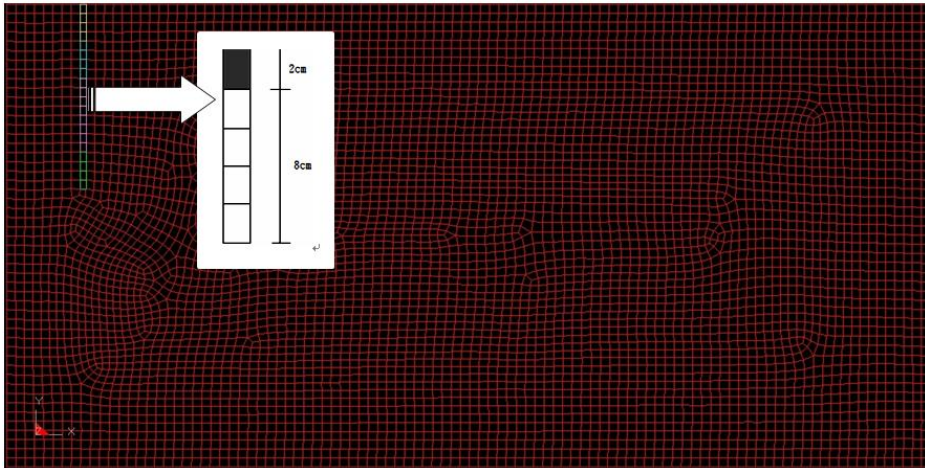


Fig. 3. Numerical model of stress wave with different epoxy injection condition (Labeled E2 cm_A8 cm)

The results of time-history of nodal vertical displacements with different condition of epoxy resin injection are shown in Fig.4. The amplitude of R_{max} in time-history of nodal vertical displacement waveform is used as the parameter for investigation as shown in Fig. 5. The time-histories of nodal vertical displacements are varied with different scenarios of epoxy resin injection . Table 3 summarizes the maximum displacement of waveform (R_{max}) for all the cases. The maximum value of nodal vertical displacements (R_{max}) is obtained from the non-crack case. The R_{max} of non-crack case(E0 cm_C10 cm) from the Table 3 can be compared with the fully epoxy injected case(E10 cm_A0 cm), the value of R_{max} attenuation was 11.4%. In addition, the maximum displacement of the R-wave decreases with the decrease of length of epoxy resin injection.. Loss percentage of R_{max} with different epoxy resin injection condition was shown in Fig.6.

Table 3. Maximum displacement of waveform with different epoxy resin injection condition

Code	Maximum displacement of waveform (R_{max}), m	Loss percentage of R_{max} (%) (Base on E0 cm_C10 cm)	Remark
E0 cm_A10 cm	-5.90E-05	72.0	Non- epoxy injection case
E2 cm_A8 cm	-1.09E-04	48.3	Partial epoxy injection case
E4cm_A6 cm	-1.46E-04	30.8	Partial epoxy injection case
E6 cm_A4 cm	-1.76E-04	16.6	Partial epoxy injection case
E8 cm_A2 cm	-1.86E-04	11.8	Partial epoxy injection case
E10 cm_A0 cm	-1.87E-04	11.4	Fully epoxy injection case
E0 cm_C10 cm	-2.11E-04	0.0	Non-crack case

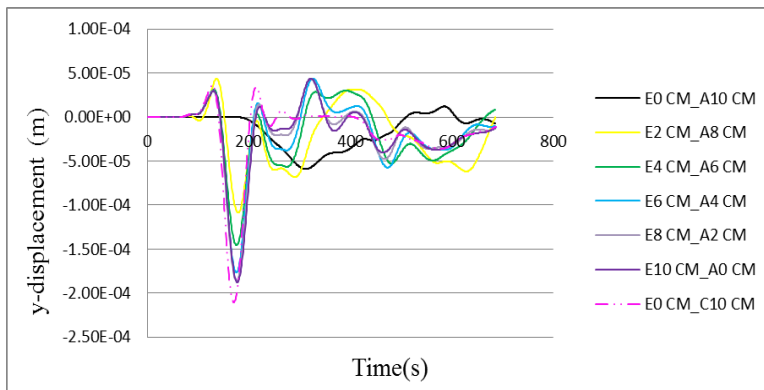


Fig. 4. Time-history of nodal vertical displacements with different epoxy resin injection condition

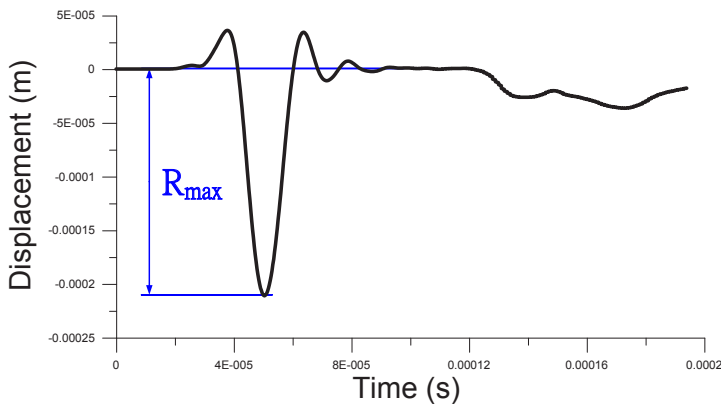


Fig. 5. The length of R_{max} in time-history of nodal vertical displacement waveform

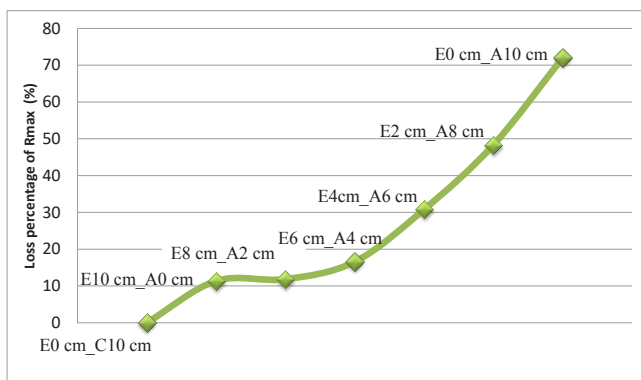


Fig. 6. Loss percentage of R_{max} with different epoxy resin injection condition

4 CONCLUSIONS

Numerical simulation results show that for surface opening crack partially filled by epoxy resin, the maximum displacement of the R-wave can be served as a parameter to evaluate the different injection condition of crack. The maximum displacement of the R-wave decreases with the decrease of the length of epoxy resin in the crack. The technique has a great potential for evaluation of the repair quality of concrete structural cracks injected by epoxy resin.

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