

Seismic Fragility Curves of Industrial Buildings by Using Nonlinear Analysis

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Abstract. This study presents the steel fragility curves and performance curves of industrial buildings of different geometries. The fragility curves were obtained for different building geometries, and the performance curves were developed based on lateral load, which is affected by the geometry of the building. Three records of far-field ground motion were used for incremental dynamic analysis (IDA), and the design lateral loads for pushover analysis (POA). All designs were based on British Standard (BS 5950); however, Eurocode 8 was preferred for seismic consideration in the analysis because BS 5950 does not specify any seismic provision. The five levels of performance stated by FEMA-273, namely, operational phase, immediate occupancy, damage control, life safety, and collapse prevention (CP) were used as main guidelines for evaluating structural performance. For POA, Model 2 had highest base shear, followed by Model 1 and Model 3, even though Model 2 has a smaller structure compared with Model 3. Meanwhile, the fragility curves showed that the probability of reaching or exceeding the CP level of Model 2 is the highest, followed by that of Models 1 and 3.

1 Introduction

Structures need to be well designed and constructed to reduce damage. Industrial structures are commonly composed of prefabricated structures or steel because of their lightweight features and the maximum span between columns must not exceed 10 m. Damage on industrialized structures is commonly caused by brace buckling [1], which can be assessed based on the performance level of drift. Drift is commonly used as a damage parameter to predict the performance of industrialized buildings.

Drift has primary effects caused by the movement of structural elements, non-structural elements, and adjacent structures [2]. Drift can be described by considering the maximum allowable damage state to identify the performance level [3]. The performance level is classified according to FEMA 273 [4] and FEMA-356 [5] guidelines. These guidelines describe four types of performance levels, namely, fully operational (damage is

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negligible and facilities continue their operations), operational (minor damage and minor disruption, facilities continue their operations), life safe (damage is moderate to extensive, lives are slightly at risk), and near collapse (structural collapse is impeded, lives are at risk).

Fragility can be used as a tool for predicting structural and non-structural damage [6]. The fragility curve expresses the probability of a structure exceeding or reaching its damage state [7-13], and allows for estimating the structural damage level for a known ground motion index. The current study has two main objectives: (i) to examine the drift of a structure based on different industrial building designs and (ii) to develop the seismic fragility curve for industrial structures based on different geometries.

2 Structural geometry

Three different types of geometry were applied in this study. Geometry is based on the typical types of industrial buildings in Malaysia, as shown in Fig. 1. All models are 6 m in height with different numbers of bays. Table 1 presents the summary of the industrial building models in this study.

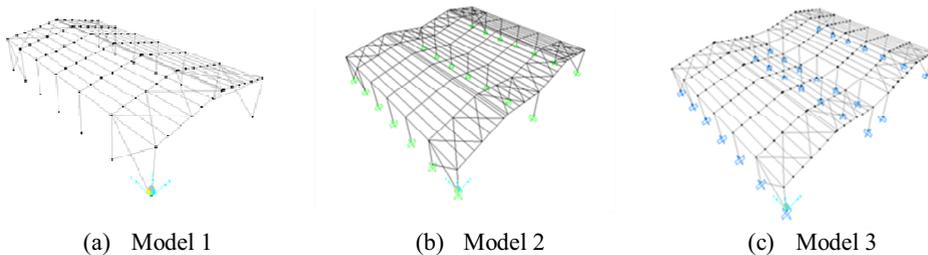


Fig. 1. 3D model with different geometry.

Table 1. Summary of industrial building models.

Frame	nb	Lb [m]	Lc [m]	Beam Section	Column Section
1	1	6	6	UB 533x210x82	UC 356x168x129
2	2	6	6	UB 406x140x46	UC 305x305x198
3	3	6	6	UB 356x171x151	UB 457x152x60

The live loads are 1 kN/m and dead loads are 2 kN/m. The models are assumed suited at soil type A, which is considered stable rock, such as geological formations with least 5 m of weaker material at the surface. Soil type A was used in this study to avoid soil–structure interaction in the analysis. SAP 2000 was used as the main tool to analyze the two types of practice-oriented methods, namely, pushover analysis (POA) and incremental dynamic analysis (IDA).

Several aspects need to be defined to perform POA, including material information, frame dimension, load patterns, load cases, frame hinge properties, and joint constraint [14]. These aspects must be assigned on the model of the existing industrial building for the analysis. All connections must be assigned correctly to avoid error. In this study, models were introduced in uniform distribution.

The selection of ground motion records is important for IDA. An appropriate number of recordings is a significant concern [15-16]. However, most codes [5, 18-19] recommend a minimum of three or seven sets of ground motion records. For this study, three sets of ground motions were used and selected from the PEER NGA website (Table 2).

Table 2. Selected ground motions records

Name Record	Earthquake Location	Year	Magnitude
Chichi	Chichi, Taiwan	1975	7.8
Kobe	Kobe, Japan	1975	6.8
Tehri	Tehri, India	1979	6.8

Far-field (FF) ground motions were chosen based on the following criteria: (i) Joyner–Boore distance is more than 20 km and (ii) magnitude ranges from 7 to 8. These ground motions were then scaled to elastic response spectra. The elastic response spectra were developed from 0 g to 6 g (an increment of 2 g).

To develop a fragility curve, the IDA curve with the displacement or drift of a structure must be obtained. The fragility curve can be plotted using the suggested limit condition in PGA–drift format form [20]. Equation (1), as suggested by Ibrahim and El-Shami [6] was used in this study to develop the fragility curve.

$$P[D/PGA]=\Phi((\ln(PGA)-\mu)/\sigma) \quad (1)$$

where ;

D = damage

PGA = Peak Ground Acceleration

Φ = standard normal cumulative distribution

μ = mean

σ = standard deviation of the natural logarithm of PGA

3 Results and discussion

The results obtained from POA and IDA was evaluated by comparing them with the limit state based on PBSB. Five limit states were used, namely, operational phase (OP) with 0.5% drift, immediate occupancy (IO) with 1.0% drift, damage control (DC) with 1.5% drift, life safety (LS) with 2.0% drift, and collapse prevention (CP) with more than 2.5% drift.

3.1 Seismic performance

A base shear versus drift graph can be plotted from the POA results. This performance curve can describe the maximum allowable damage condition for a known seismic hazard. Fig. 2 shows the performance curves for the three models.

Based on Fig. 3, at 0.5% drift, the base shears of Model 2 and 3 were similar, but a small difference was found between the base shears of Model 1 and Model 2 (i.e., 14%). At 1.1% drift, the performance curves of Model 2 and 3 start to diverge. At the CP level, a 16% difference between Model 1 and Model 2 was observed at 2.5% drift. Meanwhile, the difference between Model 2 and Model 3 at 2.5% drift was approximately 21%. Although Model 3 is larger, Model 2 reaches a base shear that is greater than that of Model 3. This difference is probably because the structure of Model 3 is composed of smaller members. The column on Model 3 cannot resist the lateral load exerted on the structure, resulting in a low ability to resist the lateral load applied.

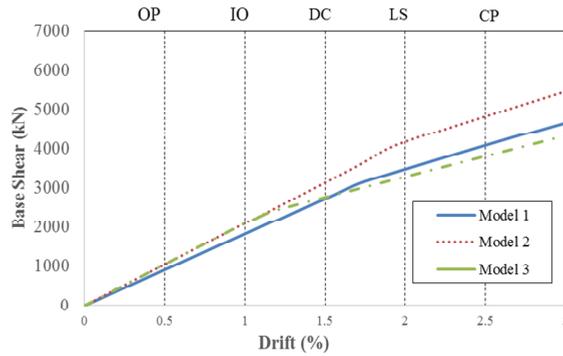


Fig. 2. Seismic performance.

3.2 Hinge deformation

Figure 3 shows the hinge deformation for all models that based on the IDA. The most common failure that occurs in Model 1 and Model 2 is at the column member. For Model 3, failure occurs at the truss and column section. From the hinge deformation result, Model 3 has the worst damage because Model 3 reaches Level 7. The most severe damage occurs at the column, which may be because the structure uses small sections for columns (i.e., UB547×152×60) compared with other models using UC section. Model 2 has a greater damage level than Model 1 because it reached level 5, which is the collapse level. Collapse may occur because of the greater size of the structure of Model 2 than Model 1. In addition, the damage may be due to connection failures, given that Model 2 has no detailed design.

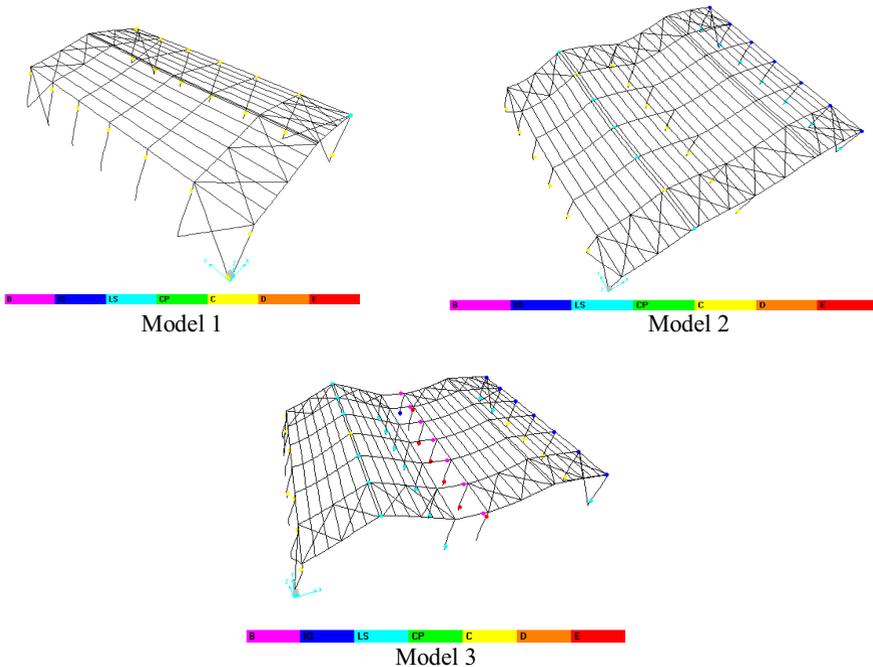


Fig. 3. Hinge deformation

3.3 IDA

IDA was conducted under each ground motion and the scaling PGA increased every 2 g until reaching 6 g. Normally, the analysis would be stopped when PGA reaches 6 g because structures are commonly unstable at 6 g. Every structure has different behavior toward ground motion, and thus the ground motion records have different patterns of IDA curves. An IDA curve shows the relationship between drift and PGA. In this study, drift was only emphasized until 3% to develop the seismic fragility curve; 3% is the maximum drift limit according to FEMA-273. The mean drift was calculated for every PGA to present the average of the IDA curves and was compared with the limit state, as shown in Fig. 4.

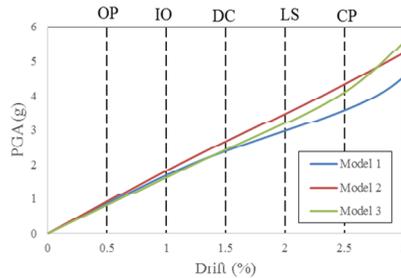


Fig. 4. Mean IDA.

Based on the observation at the CP levels, Model 2 has a better performance than Model 1 and 3, because the members of Model 2 are strong enough to for the ground motion. The larger structural members of Model 2 endow stability when ground motion occurs. The mean difference between Model 2 and 3 is only about 5%. Although Model 3 is larger than Model 2, Model 3 cannot resist the load applied on the structure because the compression members of Model 3 are smaller (UB section). If Model 3 were designed with a UC section at the compression member, then Model 3 would have inferior performance to Model 2. In terms of structural size (whole), Model 3 is the largest, followed by Model 2 and Model 1. Among the models, Model 1 has the lowest performance. The mean difference between Model 1 and Model 2 is about 17%, which might be due to the design and size of the structure. Thus, Model 1 will collapse before Model 2 and Model 3.

3.4 Fragility curve

Two parameters, namely, mean and standard deviation, are needed to develop a fragility curve. The mean and standard deviation of PGA were calculated for every point in the IDA curve, which were across the vertical gridlines of limit state at drifts. The calculated parameters are provided in Table 3. The fragility curves were developed using (1). All seismic fragilities were represented in the damage probability curves, as shown in Fig. 5.

Table 3. Parameters of log-normal distribution of the models.

Model	OP		IO		DC		LS		CP	
	μ	σ	μ	σ	μ	σ	μ	σ	μ	σ
1	-0.009	0.700	0.677	0.688	0.689	0.201	0.938	0.253	1.136	0.334
2	-1.332	1.372	0.673	0.871	0.6109	0.0745	0.870	0.0304	1.201	0.131
3	-0.041	0.102	0.607	0.91	0.837	0.361	1.256	0.110	1.512	0.121

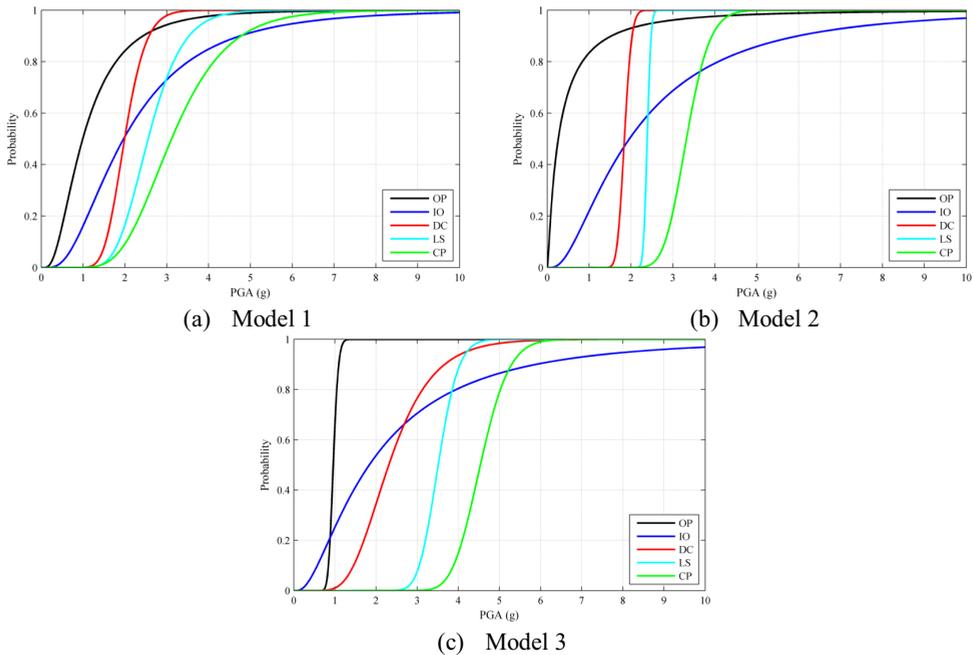


Fig. 5. Seismic fragility curves

As shown in Fig. 4(a), at FF ground motion of 1g, the probabilities of reaching or exceeding the OP and IO levels are 79% and 26%, respectively. At a ground motion of 5 g, the probabilities of reaching or exceeding the CP and IO levels are approximately 92% and 90%, respectively, and the probabilities of exceeding or reaching the OP, LS, and DC levels are almost 100%. The following observations were noticed for Model 2 (Fig. 4(b)): when a PGA of 1 g was exerted, the probabilities of reaching OP and IO levels were 84% and 22%, respectively. When PGA was 5 g, the probability of reaching the DC, LS, and CP levels were 100%. The probabilities of exceeding or reaching the OP and IO levels were 98% and 90%, respectively. Fig 4(c) shows the fragility curve of Model 3. At a PGA of 1 g, the probabilities of exceeding or reaching the OP and IO levels are 60% and 25%, respectively. At a strong ground motion of 5 g, the probabilities of exceeding the OP, IO, DC, LS, and CP levels are 100%, 86%, 98%, 100%, and 78%, respectively.

4 Conclusions

This study develops the fragility curve of industrial buildings based on the practice-oriented methods, namely, POA and IDA. The models were designed based on BS 5950; however, they were analyzed according to Eurocode 8 because BS 5950 does not specify any seismic provision. For the analysis, SAP 2000 software was used as the main tool for analyzing the structure under static load (lateral load) and dynamic load (ground motion). Three sets of ground motion records around Asia were chosen, which were classified as FF. The drift of the structure from POA and IDA were investigated. The following conclusions can be drawn from this study.

- i) The performance curves were developed using SAP 2000 software. Lateral load exerted on the structure varies depending on the structure. The structure with a greater mass can resist higher base forces. However, the drift of the structure also depends on the design of the industrial building. For instance, although Model 3 has a larger surface area than Model 2, Model 3 collapses 21% faster than Model 2. Although Model 3 is larger,

Model 2 reaches a base shear that is greater than Model 3. This result is most probably because Model 3 consists of smaller structural members, and the strength of Model cannot resist the load applied to the structure. The different analyses provided different drifts, even for the same model.

- ii) The fragility curves were developed for different types of geometry. The fragility curves were analyzed based on five limit states, namely, OP, IO, DC, LS, and CP. By considering the limit states, designers can predict the damage level and possible location of hinge deformation. Comparison of the fragility curves of the three models show that Model 2 has the highest probability of reaching OP and CP levels.

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